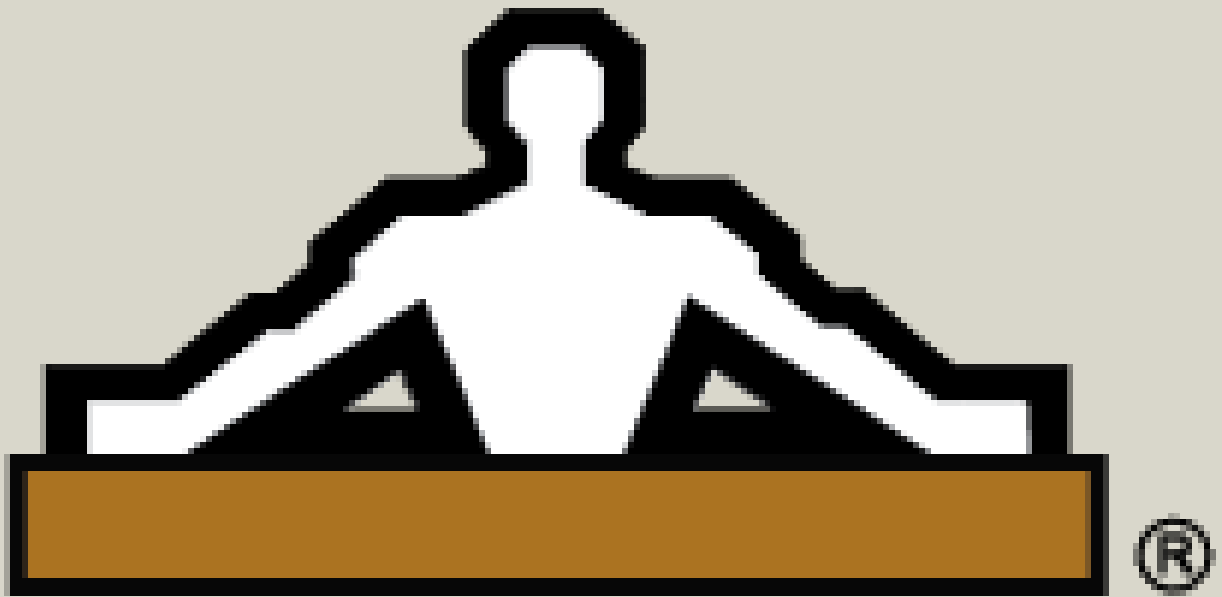




MASONITE BEAMS



The mark of
responsible forestry
Please contact us for our
FSC® certified products

Apartment Span Tables

Apartment Floor Span Table

Dead = 1.0kN/m²; Partition = 0.35kN/m²;
Live = 1.5kN/m² short-term

P5 Particle Board Web

Joist centres (mm)

Depth	Joist Type	Width	400	480	600
220	HL220	47	3820	3450	3150
	H220	47	4010	3730	3400
	HM220	60	4320	4010	3650
	HI220	70	4530	4200	3820
	HB220	97	4920	4620	4190
240	HL240	47	4130	3840	3510
	H240	47	4330	4030	3680
	HM240	60	4660	4340	3950
	HI240	70	4830	4540	4140
	HB240	97	5200	4940	4540
300	HL300	47	4860	4640	4260
	H300	47	5050	4800	4470
	HM300	60	5340	5080	4780
	HI300	70	5530	5260	4940
	HB300	97	5960	5660	5320
350	H350	47	5330	5080	4780
	HM350	60	5840	5560	5230
	HI350	70	6060	5870	5420
	HB350	97	6520	6200	5820
400	H400	47	5980	5700	5030
	HM400	60	6320	6020	5300
	HI400	70	6550	6230	5830
	HB400	97	7040	6700	6290

DESIGN ASSUMPTIONS:

1. Spans are calculated for uniform loading of: 0.4kN/m² dead; 0.35kN/m² partitions ; 1.50kN/m² imposed load
2. Loading allows for a 22mm chipboard deck and direct applied 15mm plasterboard ceiling
3. Local resistance has been verified under 2.0 kN imposed concentrated load in accordance with BS EN 1991-1-1, §6.2.1.3(P)
4. Deflections: **w_{inst}** is limited to L/333 or 12mm whichever is the lowest ; **w_{net,fin}** is limited to L/250
5. Vibration design in accordance with UK NA to BS EN 1995-1-1 dated 2012-11
6. Bearing resistance assuming: 45mm end bearing ; 89mm intermediate bearing ; no web stiffeners ; no concentrated load above
7. Resistances assume loadsharing i.e. 4 or more joists at no more than 610 centres (k_{sys} = 1.10)
8. Spans listed are worst case between simple and continuous span situation (the latter assuming two equal clear spans)

Apartment Floor Span Table

Dead = 1.0kN/m²; Partition = 0.35kN/m²;
Live = 1.5kN/m² short-term

Depth	OSB Web	Joist centres (mm)			
	Joist Type	Width	400	480	600
220	HL220	47	3920	3660	3360
	H220	47	4130	3850	3530
	HM220	60	4430	4150	3800
	HI220	70	4590	4350	3980
	HB220	97	4950	4710	4390
240	HL240	47	4230	3950	3620
	H240	47	4420	4150	3810
	HM240	60	4680	4460	4110
	HI240	70	4850	4620	4300
	HB240	97	5230	4970	4670
300	HL300	47	4780	4650	4380
	H300	47	5070	4830	4550
	HM300	60	5370	5110	4810
	HI300	70	5560	5300	4980
	HB300	97	5990	5700	5360
350	H350	47	5560	5290	4980
	HM350	60	5880	5600	5270
	HI350	70	6090	5800	5450
	HB350	97	6560	6240	5870
400	H400	47	6010	5720	5230
	HM400	60	6350	6050	5510
	HI400	70	6590	6270	5900
	HB400	97	7090	6740	6340
450	HB450	97	7580	7210	6780
500	HB500	97	8050	7660	6920

DESIGN ASSUMPTIONS:

1. Spans are calculated for uniform loading of: 1.0kN/m² dead; 0.35kN/m² partitions ; 1.50kN/m² imposed load
2. Loading allows for a 22mm chipboard deck and direct applied 15mm plasterboard ceiling
3. Local resistance has been verified under 2.0 kN imposed concentrated load in accordance with BS EN 1991-1-1, §6.2.1.3(P)
4. Deflections: **w_{inst}** is limited to L/333 or 12mm whichever is the lowest ; **w_{net,fin}** is limited to L/250
5. Vibration design in accordance with UK NA to BS EN 1995-1-1 dated 2012-11
6. Bearing resistance assuming: 45mm end bearing ; 89mm intermediate bearing ; no web stiffeners ; no concentrated load above
7. Resistances assume loadsharing i.e. 4 or more joists at no more than 610 centres (**k_{sys}** = 1.10)
8. Spans listed are worst case between simple and continuous span situation (the latter assuming two equal clear spans)